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The analytical solutions for orthotropic cantilever beams (I): Subjected to surface forces^{*}

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Abstract: This paper first gives the general solution of two-dimensional orthotropic media expressed with two harmonic displacement functions by using the governing equations. Then, based on the general solution in the case of distinct eigenvalues, a series of beam problems, including the problem of cantilever beam under uniform loads, cantilever beam with axial load and bending moment at the free end, cantilever beam under the first, second, third and fourth power of *x* tangential loads, is solved by the superposition principle and the trial-and-error methods.

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INTRODUCTION

The problem of cantilever beams subjected to uniform loads is a classic one in elasticity studies. Timoshenko and Goodier (1970) presented a solution for an isotropic cantilever beam subjected to uniform load and cross load at free end. Lekhnitskii (1969) obtained analytical solutions for an orthotropic cantilever beam subjected to cross load at free end and uniform load on the upper surface. The solutions for constant body force cases were also presented in the above two books. To the authors' knowledge, no literature about the corresponding solution of orthotropic cantilever beam with variable body forces had been published yet. The problems of density functionally graded media can be transformed into those ones with variable body forces. In order to solve the problems of variable body forces, we should first analyze the solution for cantilever beam with axial

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load and bending moment at free end, and under the normal and tangential loads on the upper and bottom surfaces.

In this paper, we will consider the orthotropic plane problems. The general solution of two-dimensional orthotropic media expressed with two harmonic displacement functions is given at first by use of the governing equations. Then, based on the general solution in the case of distinct eigenvalues, a series of beam problems, including cantilever beam under uniform loads, cantilever beam with axial load and bending moment at the free end, cantilever beam under the first, second, third and fourth power of x tangential loads, is solved by the trial-and-error methods.

Analytical solutions for various problems are obtained by the superposition principle.

GENERAL SOLUTION FOR THE PLANE PROBLEM OF ORTHOTROPIC SOLID

For the plane problems of orthotropic media, the

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displacements u_i are assumed to be independent of y for the plane-strain case. The basic equations for two-dimensional orthotropic solid in *xoz* coordinates can be simplified as follows:

$$\frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau_{xz}}{\partial z} + f_x = 0, \quad \frac{\partial \tau_{xz}}{\partial x} + \frac{\partial \sigma_z}{\partial z} + f_z = 0 \tag{1}$$

$$\sigma_{x} = c_{11} \frac{\partial u}{\partial x} + c_{13} \frac{\partial w}{\partial z}, \quad \tau_{xz} = c_{55} \left(\frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right),$$

$$\sigma_{z} = c_{13} \frac{\partial u}{\partial x} + c_{33} \frac{\partial w}{\partial z}$$
(2)

where $\sigma_x (\sigma_z, \tau_{xz})$ and u(w) are the components of stress and displacement, respectively; f_x and f_z are body force; c_{ij} are the elastic constants.

Governing Eq.(1) can be expressed in terms of u and w by virtue of Eq.(2) as follows

$$\left(c_{11}\frac{\partial^2}{\partial x^2} + c_{55}\frac{\partial^2}{\partial z^2}\right)u + (c_{13} + c_{55})\frac{\partial^2 w}{\partial x \partial z} + f_x = 0$$
(3)

$$(c_{13} + c_{55})\frac{\partial^2 u}{\partial x \partial z} + \left(c_{55}\frac{\partial^2}{\partial x^2} + c_{33}\frac{\partial^2}{\partial z^2}\right)w + f_z = 0 \qquad (4)$$

Ding *et al.*(1997a; 1997b) derived the general solution for piezoelectric plane problem without body forces, in which all physical quantities are expressed in three harmonic functions. With the method and the strict differential operator theorem presented in Ding *et al.*(1997a; 1997b), the general solution of two-dimensional orthotropic media without body forces in the case of distinct eigenvalues can be easily derived and expressed in two harmonic functions as follows

$$u = \sum_{j=1}^{2} \frac{\partial \psi_{j}}{\partial x}, w = \sum_{j=1}^{2} s_{j} k_{j} \frac{\partial \psi_{j}}{\partial z_{j}}, \sigma_{x} = \sum_{j=1}^{2} \omega_{2j} \frac{\partial^{2} \psi_{j}}{\partial z_{j}^{2}},$$
$$\sigma_{z} = \sum_{j=1}^{2} \omega_{1j} \frac{\partial^{2} \psi_{j}}{\partial z_{j}^{2}}, \tau_{xz} = \sum_{j=1}^{2} s_{j} \omega_{1j} \frac{\partial^{2} \psi_{j}}{\partial x \partial z_{j}}$$
(5)

where the functions ψ_j satisfy the following equations:

$$\left(\frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial z_j^2}\right)\psi_j = 0, \ (j=1,2)$$
(6)

where $z_j = s_j z$ (j=1,2) and s_j^2 are the two roots of the equation [we take $\operatorname{Re}(s_i) > 0$]

$$a_1 s^4 - a_2 s^2 + a_3 = 0 \tag{7}$$

where

$$a_1 = c_{33}c_{44}, \ a_2 = c_{11}c_{33} + c_{55}^2 - (c_{13} + c_{55})^2, \ a_3 = c_{11}c_{55}$$
(8a)

$$k_{j} = \frac{-c_{11} + c_{55}s_{j}^{2}}{-(c_{13} + c_{55})s_{j}^{2}}, \ \omega_{1j} = c_{33}s_{j}^{2}k_{j} - c_{13},$$
$$\omega_{2j} = -s_{j}^{2}\omega_{1j}, \qquad (j=1,2)$$
(8b)

The polynomials listed in Appendix A can be chosen as harmonic functions ψ_j simply by replacing z with z_j . In the next sections, we will consider three loads cases of cantilever beam shown in Fig.1, and derive the analytical solutions by using the general solution (5).



Fig.1 The geometry and coordinate system of a cantilever beam

THREE SOLUTIONS FOR CANTILEVER BEAM WITHOUT BODY FORCES

Cantilever beam under uniform loads on the upper and bottom surfaces

We introduce the displacement function as follows

$$\psi_{j} = (x^{2} - z_{j}^{2})A_{2j} + (x^{2}z_{j} - \frac{1}{3}z_{j}^{3})B_{3j} + B_{5j}(x^{4}z_{j} - 2x^{2}z_{j}^{3} + \frac{1}{5}z_{j}^{5})$$
(9)

where A_{2j} , B_{3j} and B_{5j} (j=1,2) are unknown constants to be determined.

Substituting Eq.(9) into Eq.(5) leads to

$$u = \sum_{j=1}^{2} \left[2xA_{2j} + 2xz_{j}B_{3j} + (4x^{3}z_{j} - 4xz_{j}^{3})B_{5j} \right]$$
(10a)

$$w = \sum_{j=1}^{2} s_{j} k_{j} [-2z_{j} A_{2j} + (x^{2} - z_{j}^{2}) B_{3j} + (x^{4} - 6x^{2} z_{j}^{2} + z_{j}^{4}) B_{5j}]$$
(10b)

$$\sigma_{z} = \sum_{j=1}^{2} \omega_{1j} [-2A_{2j} - 2z_{j}B_{3j} + (-12x^{2}z_{j} + 4z_{j}^{3})B_{5j}]$$
(10c)

$$\tau_{xz} = \sum_{j=1}^{2} s_{j} \omega_{1j} [2xB_{3j} + (4x^{3} - 12xz_{j}^{2})B_{5j}]$$
(10d)

$$\sigma_x = \sum_{j=1}^{2} \omega_{2j} \left[-2A_{2j} - 2z_j B_{3j} + (-12x^2 z_j + 4z_j^3) B_{5j} \right]$$
(10e)

The boundary conditions are

$$z = \pm h/2: \sigma_z = \beta_1 \pm C_1, \quad \tau_{xz} = 0$$
(11a)

$$x = 0: \int_{-h/2}^{h/2} \sigma_x dz = 0, \int_{-h/2}^{h/2} \sigma_x z dz = 0, \int_{-h/2}^{+h/2} \tau_{xz} dz = 0$$
(11b)
$$(x = L, z = 0): u = 0, w = 0, \partial w / \partial x = 0$$
(11c)

Substituting Eqs.(10c), (10d) and (10e) into Eqs.(11a) and (11b), we arrive at

$$\sum_{j=1}^{2} \omega_{1j} A_{2j} = -\beta_1 / 2, \ \sum_{j=1}^{2} \omega_{1j} (-hs_j B_{3j} + \frac{1}{2} h^3 s_j^3 B_{5j}) = C_1$$
(12)

$$\sum_{j=1}^{2} s_{j} \omega_{1j} B_{5j} = 0, \qquad \sum_{j=1}^{2} \omega_{2j} A_{2j} = 0$$
(13)

$$\sum_{j=1}^{2} s_{j} \omega_{1j} (2B_{3j} - 3h^{2} s_{j}^{2} B_{5j}) = 0$$
(14)

$$\sum_{j=1}^{2} s_{j} \omega_{2j} (-10B_{3j} + 3h^{2} s_{j}^{2} B_{5j}) = 0$$
(15)

Then, the unknown constants A_{2j} , B_{3j} and B_{5j} (*j*=1,2) can be determined from Eqs.(12)–(15). To satisfy the boundary conditions Eq.(11c), the solution above should be superposed on the rigid body displacements solutions as follows

$$u_1 = u_0 + \omega_0 z, \qquad w_1 = w_0 - \omega_0 x$$
 (16)

where

$$u_0 = -2L \sum_{j=1}^2 A_{2j}, \ \omega_0 = 2L \sum_{j=1}^2 s_j k_j (B_{3j} + 2L^2 B_{5j}) \ (17a)$$

$$w_0 = L^2 \sum_{j=1}^2 s_j k_j (B_{3j} + 3L^2 B_{5j})$$
(17b)

Cantilever beam with axial force N and bending moment M at the free end

We constitute the displacement function as follows

$$\psi_j = (x^2 - z_j^2)A_{2j} + (x^2 z_j - \frac{1}{3}z_j^3)B_{3j}, \quad (j = 1, 2) \quad (18)$$

Substituting Eq.(18) into Eq.(5) leads to

$$u = \sum_{j=1}^{2} (2xA_{2j} + 2xz_{j}B_{3j}),$$

$$w = \sum_{j=1}^{2} s_{j}k_{j}[-2z_{j}A_{2j} + (x^{2} - z_{j}^{2})B_{3j}]$$
(19a)

$$\sigma_{z} = \sum_{j=1}^{2} \omega_{1j}(-2A_{2j} - 2z_{j}B_{3j}), \quad \tau_{xz} = 2x\sum_{j=1}^{2} s_{j}\omega_{1j}B_{3j}$$

$$\sigma_x = \sum_{j=1}^{2} \omega_{2j} (-2A_{2j} - 2z_j B_{3j})$$
(19c)

The boundary conditions are

$$z = \pm h/2: \sigma_z = 0, \quad \tau_{xz} = 0 \tag{20a}$$

$$x = 0: \int_{-h/2}^{h/2} \sigma_x dz = N, \ \int_{-h/2}^{h/2} \sigma_x z dz = M, \ \int_{-h/2}^{h/2} \tau_{xz} dz = 0$$
(20b)

$$(x = L, z = 0): u = 0, w = 0, \partial w / \partial x = 0$$
 (20c)

Substituting Eqs.(19b) and (19c) into Eqs.(20a) and (20b), we have

$$\sum_{j=1}^{2} \omega_{1j} A_{2j} = 0, \qquad \sum_{j=1}^{2} s_j \omega_{1j} B_{3j} = 0$$
(21)

$$-\frac{h^3}{6}\sum_{j=1}^2 s_j \omega_{2j} B_{3j} = M, \quad -2h\sum_{j=1}^2 \omega_{2j} A_{2j} = N$$
(22)

Then, the constants A_{2j} and B_{3j} can be determined from Eqs.(21) and (22). To satisfy the boundary conditions Eq.(20c), the solution above should be superposed on the rigid body displacement solutions as follows (23)

$$u_1 = u_0 + \omega_0 z, \qquad w_1 = w_0 - \omega_0 x$$

where

$$u_{0} = -2L\sum_{j=1}^{2} A_{2j}, \omega_{0} = 2L\sum_{j=1}^{2} s_{j}k_{j}B_{3j}, w_{0} = L^{2}\sum_{j=1}^{2} s_{j}k_{j}B_{3j}$$
(24)

Cantilever beam with the *n*th power of *x* tangential loads on the upper and bottom surfaces

The boundary conditions are taken as

$$z = \pm h/2 : \sigma_z = 0, \quad \tau_{xz} = T_n x^n \tag{25a}$$

$$x = 0: \int_{-h/2}^{h/2} \sigma_x dz = 0, \ \int_{-h/2}^{h/2} \sigma_x z dz = 0, \ \int_{-h/2}^{h/2} \tau_{xz} dz = 0$$
(25b)

$$(x = L, z = 0): u = 0, w = 0, \partial w / \partial x = 0$$
 (25c)

We introduce the displacement function as follows

$$\psi_{j} = B_{2j} \varphi_{2}^{1}(x, z_{j}) + B_{4j} \varphi_{4}^{1}(x, z_{j}) + \dots + B_{n+4,j} \varphi_{n+4}^{1}(x, z_{j})$$

$$(j = 1, 2; n = 2, 4, 6, \dots)$$
(26a)

$$\psi_{j} = B_{3j}\varphi_{3}^{1}(x, z_{j}) + B_{5j}\varphi_{5}^{1}(x, z_{j}) + \dots + B_{n+4,j}\varphi_{n+4}^{1}(x, z_{j})$$

$$(j = 1, 2; n = 1, 3, 5, \dots)$$
(26b)

where B_{mj} are undetermined constants, and $\varphi_m^1(x, z_j)$ are taken from Appendix A.

Substituting Eq.(26) into Eq.(5) leads to the expressions of displacements and stresses. When n is an even number, we have

$$u = \sum_{j=1}^{2} [z_{j}B_{2j} + (3x^{2}z_{j} - z_{j}^{3})B_{4j} + (5x^{4}z_{j} - 10x^{2}z_{j}^{3} + z_{j}^{5})B_{6j} + (7x^{6}z_{j} - 35x^{4}z_{j}^{3} + 21x^{2}z_{j}^{5} - z_{j}^{7})B_{8j} + \cdots]$$
(27a)

$$w = \sum_{j=1}^{2} s_{j}k_{j}[xB_{2j} + (x^{3} - 3xz_{j}^{2})B_{4j} + (x^{5} - 10x^{3}z_{j}^{2} + 5xz_{j}^{4})B_{6j} + (x^{7} - 21x^{5}z_{j}^{2} + 35x^{3}z_{j}^{4} - 7xz_{j}^{6})B_{8j} + \cdots]$$
(27b)

$$\sigma_{z} = \sum_{j=1}^{2} \omega_{1j} [-6xz_{j}B_{4j} + 20xz_{j}(z_{j}^{2} - x^{2})B_{6j} + (-42x^{5}z_{j} + 140x^{3}z_{j}^{3} - 42xz^{5})B_{2j} + 140x^{3}z_{j} + 140x^{3}z_{j}^{3} - 42xz^{5})B_{2j} + 140x^{3} + 140x^{3} + 140x^{3} + 140x^{3} + 140x^{3} + 140x^{3} + 14$$

$$+140x^{3}z_{j}^{3} - 42xz_{j}^{3})B_{8j} + \cdots]$$
(27c)
$$\tau_{xz} = \sum_{j=1}^{2} s_{j}\omega_{1j} [B_{2j} + (3x^{2} - 3z_{j}^{2})B_{4j} + (5x^{4} - 30x^{2}z_{j}^{2})]$$

$$+5z_{j}^{4})B_{6j} + (7x^{6} - 105x^{4}z_{j}^{2} + 105x^{2}z_{j}^{4} - 7z_{j}^{6})B_{8j} + \cdots]$$
(27d)

$$\sigma_{x} = \sum_{j=1}^{2} \omega_{2j} [-6xz_{j}B_{4j} + 20xz_{j}(z_{j}^{2} - x^{2})B_{6j} + (-42x^{5}z_{j} + 140x^{3}z_{j}^{3} - 42xz_{j}^{5})B_{8j} + \cdots]$$
(27e)

When *n* is an odd number, we have

$$u = \sum_{j=1}^{2} [2xz_{j}B_{3j} + (4x^{3}z_{j} - 4xz_{j}^{3})B_{5j} + (6x^{5}z_{j} - 20x^{3}z_{j}^{3} + 6xz_{j}^{5})B_{7j} + \cdots]$$
(28a)

$$w = \sum_{j=1}^{2} s_j k_j [(x^2 - z_j^2) B_{3j} + (x^4 - 6x^2 z_j^2 + z_j^4) B_{5j} + (x^6 - 15x^4 z_j^2 + 15x^2 z_j^4 - z_j^6) B_{7j} + \cdots]$$
(28b)

$$\sigma_{z} = \sum_{j=1}^{2} \omega_{1j} [-2z_{j}B_{3j} + (-12x^{2}z_{j} + 4z_{j}^{3})B_{5j} + (-30x^{4}z_{j} + 60x^{2}z_{j}^{3} - 6z_{j}^{5})B_{7j} + \cdots]$$
(28c)

$$\sigma_{x} = \sum_{j=1}^{2} \omega_{2j} [-2z_{j}B_{3j} + (-12x^{2}z_{j} + 4z_{j}^{3})B_{5j} + (-30x^{4}z_{j} + 60x^{2}z_{j}^{3} - 6z_{j}^{5})B_{7j} + \cdots]$$
(28d)

$$\tau_{xz} = \sum_{j=1}^{2} s_j \omega_{1j} [2xB_{3j} + 4(x^3 - 3xz_j^2)B_{5j} + 6(x^5 - 10x^3z_j^2 + 5xz_j^4)B_{7j} + \cdots]$$
(28e)

When n=1, we substitute Eqs.(28c), (28d) and (28e) into Eqs.(25a) and (25b) and have

$$\sum_{j=1}^{2} \omega_{1j} (-hs_{j}B_{3j} + \frac{1}{2}h^{3}s_{j}^{3}B_{5j}) = 0, \sum_{j=1}^{2} s_{j}\omega_{1j}B_{5j} = 0 \quad (29)$$

$$\sum_{j=1}^{2} s_{j}\omega_{1j} (2B_{3j} - 3h^{2}s_{j}^{2}B_{5j}) = T_{1},$$

$$\sum_{j=1}^{2} s_{j}\omega_{2j} (-10B_{3j} + 3h^{2}s_{j}^{2}B_{5j}) = 0 \quad (30)$$

Then, the unknown constants B_{3j} and B_{5j} can be determined from Eqs.(29) and (30). To satisfy the boundary conditions in Eq.(25c), the solution above should be superposed on the rigid body displacements solutions as follows

$$u_1 = \omega_0 z, \qquad w_1 = w_0 - \omega_0 x$$
 (31)

where

$$\omega_{0} = 2L \sum_{j=1}^{2} s_{j} k_{j} (B_{3j} + 2L^{2} B_{5j}),$$

$$w_{0} = L^{2} \sum_{j=1}^{2} s_{j} k_{j} (B_{3j} + 3L^{2} B_{5j})$$
(32)

When n=3, we substitute Eqs.(28c), (28d) and (28e) into Eqs.(25a) and (25b) and have

$$\sum_{j=1}^{2} \omega_{1j} \left(-6hs_{j}B_{5j} + \frac{15}{2}h^{3}s_{j}^{3}B_{7j} \right) = 0, \quad \sum_{j=1}^{2} s_{j}\omega_{1j}B_{7j} = 0$$
(33)

$$\sum_{j=1}^{2} \omega_{1j} \left(-s_j h B_{3j} + \frac{1}{2} s_j^3 h^3 B_{5j} - \frac{3}{16} h^5 s_j^5 B_{7j} \right) = 0 \quad (34)$$

$$\sum_{j=1}^{2} s_{j} \omega_{1j} (4B_{5j} - 15s_{j}^{2}h^{2}B_{7j}) = T_{3}$$
(35)

$$\sum_{j=1}^{2} s_{j} \omega_{1j} \left(2B_{3j} - 3h^{2} s_{j}^{2} B_{5j} + \frac{15}{8} s_{j}^{4} h^{4} B_{7j} \right) = 0$$
(36)

$$\sum_{j=1}^{2} s_{j} \omega_{2j} \left(-\frac{1}{3} B_{3j} + \frac{1}{10} s_{j}^{2} h^{2} B_{5j} - \frac{3}{112} s_{j}^{4} h^{4} B_{7j} \right) = 0 \quad (37)$$

Then, B_{3j} , B_{5j} and B_{7j} can be determined from Eqs.(33)–(37). To satisfy the left boundary conditions in Eq.(25c), the solution above should be superposed on the rigid body displacement solutions as follows

$$u_1 = \omega_0 z, \quad w_1 = w_0 - \omega_0 x$$
 (38)

where

$$\omega_0 = 2L \sum_{j=1}^2 s_j k_j (B_{3j} + 2L^2 B_{5j} + 3L^4 B_{7j})$$
(39a)

$$w_0 = L^2 \sum_{j=1}^2 s_j k_j (B_{3j} + 3L^2 B_{5j} + 5L^4 B_{7j})$$
(39b)

When n=2, we substitute Eqs.(27c), (27d) and (27e) into Eqs.(25a) and (25b) and have

$$\sum_{j=1}^{2} \omega_{1j} \left(-3hs_{j}B_{4j} + \frac{5}{2}h^{3}s_{j}^{3}B_{6j} \right) = 0, \qquad \sum_{j=1}^{2} s_{j}\omega_{1j}B_{6j} = 0$$
(40)

$$\sum_{j=1}^{2} s_{j} \omega_{1j} \left(3B_{4j} - \frac{15}{2} h^{2} s_{j}^{2} B_{6j} \right) = T_{2}$$
(41)

$$\sum_{j=1}^{2} s_{j} \omega_{1j} \left(B_{2j} - \frac{3}{4} h^{2} s_{j}^{2} B_{4j} + \frac{5}{16} s_{j}^{4} h^{4} B_{6j} \right) = 0$$
(42)

$$\sum_{j=1}^{2} s_{j} \omega_{1j} \left(B_{2j} - \frac{1}{4} s_{j}^{2} h^{2} B_{4j} + \frac{1}{16} s_{j}^{4} h^{4} B_{6j} \right) = 0 \quad (43)$$

Substituting Eq.(27b) into the third of Eq.(25c), we have

$$\sum_{j=1}^{2} s_{j} k_{j} (B_{2j} + 3L^{2} B_{4j} + 5L^{4} B_{6j}) = 0$$
(44)

Then, the constants B_{2j} , B_{4j} and B_{6j} can be determined from Eqs.(40)–(44). To satisfy the left boundary conditions of Eq.(25c), the solution above should be superposed on the rigid body displacements solutions as follows

$$w_1 = w_0 = -L \sum_{j=1}^2 s_j k_j (B_{2j} + L^2 B_{4j} + L^4 B_{6j})$$
(45)

When n=4, we substitute Eqs.(27c), (27d) and (27e) into Eqs.(25a) and (25b) and have

$$\sum_{j=1}^{2} s_{j} \omega_{1j} B_{8j} = 0, \quad \sum_{j=1}^{2} \omega_{1j} \left(-10 s_{j} h B_{6j} + \frac{35}{2} s_{j}^{3} h^{3} B_{8j} \right) = 0$$
(46)

$$\sum_{j=1}^{2} \omega_{1j} \left(-3s_j h B_{4j} + \frac{5}{2} s_j^3 h^3 B_{6j} - \frac{21}{16} s_j^5 h^5 B_{8j} \right) = 0 \quad (47)$$

$$\sum_{j=1}^{2} s_{j} \omega_{1j} \left(5B_{6j} - \frac{105}{4} s_{j}^{2} h^{2} B_{8j} \right) = T_{4}$$
(48)

$$\sum_{j=1}^{2} s_{j} \omega_{1j} \left(3B_{4j} - \frac{15}{2} s_{j}^{2} h^{2} B_{6j} + \frac{105}{16} s_{j}^{4} h^{4} B_{8j} \right) = 0 \quad (49)$$

$$\sum_{i=1}^{2} s_{j} \omega_{1j} \left(B_{2j} - \frac{3}{4} s_{j}^{2} h^{2} B_{4j} + \frac{5}{16} s_{j}^{4} h^{4} B_{6j} - \frac{7}{64} s_{j}^{6} h^{6} B_{8j} \right) = 0$$
(50)

$$\sum_{j=1}^{2} s_{j} \omega_{1j} \left(B_{2j} - \frac{1}{4} s_{j}^{2} h^{2} B_{4j} + \frac{1}{16} s_{j}^{4} h^{4} B_{6j} - \frac{1}{64} s_{j}^{6} h^{6} B_{8j} \right) = 0$$
(51)

From Eq.(27b) and Eq.(25c), we have

$$\sum_{j=1}^{2} s_j k_j (B_{2j} + 3L^2 B_{4j} + 5L^4 B_{6j} + 7L^6 B_{8j}) = 0 \quad (52)$$

Then, B_{2j} , B_{4j} , B_{6j} and B_{8j} can be determined from Eqs.

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(46)–(52). To satisfy the left boundary conditions in Eq.(25c), the solution above should be superposed on the rigid body displacement solutions as follows

$$w_{1} = w_{0} = -L\sum_{j=1}^{2} s_{j}k_{j}(B_{2j} + L^{2}B_{4j} + L^{4}B_{6j} + L^{6}B_{8j})$$
(53)

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APPENDIX A

Harmonic polynomials for the plane problems can be written in the following form:

$$\varphi_n^m(x,z) = x^{n-m} z^m + \sum_{i=1}^{\lfloor (n-m)/2 \rfloor} (-1)^i \frac{(n-m)(n-m-1)\cdots(n-m-2i+1)}{(2i+m)!} x^{n-2i-m} z^{2i+m}$$

$$(m=0,1; n=1,2,...)$$
(A1)

where [(n-m)/2] denotes the largest integer $\leq (n-m)/2$. From Eq.(A1),the first seventeen harmonic polynomials can be written as follows:

$$\begin{split} \varphi_0^0(x,z) &= 1, \\ \varphi_1^0(x,z) &= x, \ \varphi_1^1(x,z) = z, \\ \varphi_2^0(x,z) &= x^2 - z^2, \ \varphi_2^1(x,z) = xz, \end{split}$$

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$$\begin{split} \varphi_{3}^{0}(x,z) &= x^{3} - 3xz^{2}, \ \varphi_{3}^{1}(x,z) = x^{2}z - \frac{1}{3}z^{3}, \\ \varphi_{4}^{0}(x,z) &= x^{4} - 6x^{2}z^{2} + z^{4}, \\ \varphi_{4}^{1}(x,z) &= x^{3}z - xz^{3}, \\ \varphi_{5}^{0}(x,z) &= x^{5} - 10x^{3}z^{2} + 5xz^{4}, \\ \varphi_{5}^{0}(x,z) &= x^{4}z - 2x^{2}z^{3} + \frac{1}{5}z^{5}, \\ \varphi_{6}^{0}(x,z) &= x^{6} - 15x^{4}z^{2} + 15x^{2}z^{4} - z^{6}, \\ \varphi_{6}^{1}(x,z) &= x^{5}z - \frac{10}{3}x^{3}z^{3} + xz^{5}, \\ \varphi_{7}^{0}(x,z) &= x^{7} - 21x^{5}z^{2} + 35x^{3}z^{4} - 7xz^{6}, \\ \varphi_{7}^{1}(x,z) &= x^{6}z - 5x^{4}z^{3} + 3x^{2}z^{5} - \frac{1}{7}z^{7}, \\ \varphi_{8}^{0}(x,z) &= x^{7}z - 7x^{5}z^{3} + 7x^{3}z^{5} - xz^{7} \end{split}$$
(A2)

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