



Study on a calibration equation for soil water content in field tests using time domain reflectometry*

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Abstract: The crucial point in calibrating soil water content using the technology of time domain reflectometry (TDR) is to establish the relationship between the apparent dielectric constant and the water content. Based on a database, which included 45 kinds of soil samples and 418 data points from our own test data and relevant literature, an empirical calibration equation is proposed. Additionally, the influence of soil type, dry density of soil, compaction energy, pore fluid conductivity, and temperature on the calculated result for water content was also analyzed. Results show that the equation can offer an error of ± 0.05 g/g for most soils encountered in geotechnical engineering. However, the estimation error given by the empirical equation becomes significant for soils with dry density less than 1.3 g/cm^3 , so the equation was modified to consider the influence of dry density. Both of the empirical equations can be used to test gravimetric water content using the TDR method conveniently and efficiently without calibration.

Key words: Soil, Gravimetric water content, Time domain reflectometry (TDR), Empirical calibration equation
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1 Introduction

Water content is a basic parameter of the three-phase-system of soil and affects soil behavior notably in geotechnical engineering. Obviously, therefore, it is of great significance to be able to test the water content of soil efficiently. The time domain reflectometry (TDR) method has been widely used to measure water content in agriculture, hydraulic engineering, geotechnical engineering, etc., for its advantages of speed, reliability, and the possibility of automatic monitoring (Drnevich *et al.*, 2001; Noborio, 2001; Imhoff *et al.*, 2007; Cui *et al.*, 2013; Chen, 2014).

The crucial point in calibrating soil water content using TDR technology is to establish the relationship between the dielectric constant and the water content, which is usually referred to as the calibration equation. At present there are mainly two approaches to set up the equation. One is the volumetric mixing model, which acquires the dielectric constant of a mixture by taking a weighted average of the dielectric constant of each component in the mixture according to their volumetric proportions. Since this theoretical model is usually based on some assumptions, it has limitations in practical use (Birchak *et al.*, 1974; Dobson *et al.*, 1985; Heimovaara *et al.*, 1994; Chen *et al.*, 2003). The other approach is to find a purely empirical equation to fit the experimental data points. Among these empirical equations, Topp's equation is widely used (Topp *et al.*, 1980). This equation shows that the relationship between apparent dielectric constant and volumetric water content is not sensitive to soil

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texture, soil bulk density, temperature, salt content, etc. Many studies found that this empirical equation had a high accuracy for inorganic soils but was inapplicable to organic soils, fine textural soils, and clay soils (Herkelrath *et al.*, 1991; Jacobsen and Schjønning, 1993; Dirksen and Dasberg, 1993; Ponizovsky *et al.*, 1999). There are also other types of empirical equations including a linear relationship between volumetric water content and the square root of the apparent dielectric constant (Hook and Livingston, 1996; Yu *et al.*, 1997; Masbruch and Ferré, 2003). There are also equations considering the effect of soil density (Ledieu *et al.*, 1986; Malicki *et al.*, 1996). However, in geotechnical engineering, gravimetric water content is used more extensively. According to the theory of TDR, two physical quantities, dielectric constant and bulk electrical conductivity, of soil can be obtained through the TDR waveforms. The two parameters are used in an empirical relationship relating gravimetric water content and dry density. Siddiqui and Drnevich (1995) proposed a linear calibration equation to relate dielectric constant with gravimetric water content and dry density. Then the corresponding ‘two-step method’ was performed to obtain gravimetric water content and dry density from field measurements. Yu and Drnevich (2004) established a linear empirical calibration equation to relate bulk electrical conductivity to gravimetric water content and dry density. Using this equation with Siddiqui and Drnevich (1995)’s equation, only one TDR test is required to obtain the parameters of soil *in situ*. This method is called the ‘one-step method’. Since the relationship between soil bulk electrical conductivity and gravimetric water content is not linear (Abu-Hassanein *et al.*, 1996; Zambrano, 2006), Jung (2011) proposed a ‘voltage normalization method’ and developed a calibration equation between the voltage drop parameter and the apparent dielectric constant to replace the bulk electrical conductivity equation in the ‘one-step method’. It is important that laboratory tests should be conducted to obtain the constants in the equations before using the ‘one-step method’ and the ‘two-step method’.

Generally, after calibration of the constants of the empirical equations, water content can be measured in the laboratory and in the field by the TDR method conveniently and efficiently with a high accuracy. But for the reasons listed below, there are

some constraints on applying these calibration equations in field tests:

1. The ‘two-step method’ takes time and energy to obtain the measurements for the field test. In particular, when continuous testing at different depths is required, this method will be invalid since it is difficult to perform the second TDR test.

2. For both the ‘one-step method’ and the ‘two-step method’, the constants of the calibration equations are soil-dependent. As the soil is usually heterogeneous in the field and not of a single soil type, the calibration of each soil’s constants is difficult.

Hence, it is of great significance to propose a calibration equation that is applicable to various soils and correlates gravimetric water content with apparent dielectric constant directly in geotechnical practice. The object of this study is to establish such a calibration equation through analyzing TDR data points from laboratory tests and the literature.

2 A new empirical calibration equation

Herein, the soil-water-air three phase system of soil is described according to Hook and Livingston (1996) (Fig. 1). Solid particles are considered as an impervious layer with thickness l_s and dielectric constant K_s that can have adsorbed moisture. Liquid and air phases are also treated as a liquid layer and an air layer with thicknesses l_w and l_a , and dielectric constants K_w and K , respectively. X_1 and X_2 represent the positions of the probe inserted into the soil. The relationship between the volumes of each phase is:

$$l = l_a + l_w + l_s \tag{1}$$

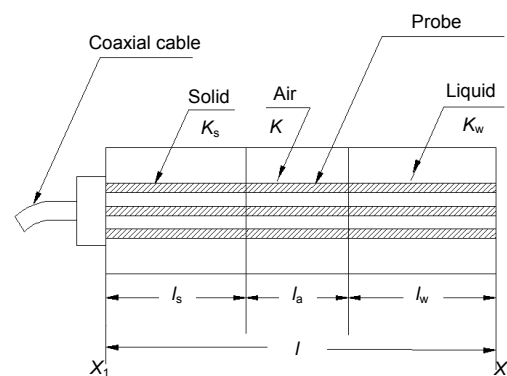


Fig. 1 Soil-water-air transmission line model (Reprinted from (Hook and Livingston, 1996), Copyright 1996, with permission from ACSESS)

It is assumed that the total travel time of the TDR waveform through the tested soil sample is equal to the sum of the time in each phase, which can be given by

$$t = t_s + t_w + t_a, \quad (2)$$

where t , t_s , t_w , and t_a represent the time of TDR waveform travel in the system, solid particles, water, and air, respectively.

Topp *et al.* (1980) showed that the velocity of an electromagnetic wave travelling through the medium could be described by an apparent dielectric constant K_a :

$$v = \frac{c}{\sqrt{K_a}}, \quad (3)$$

where v is the velocity of electromagnetic wave that travels through the medium, and c is the velocity of an electromagnetic wave in free space (the apparent dielectric constant is also called the dielectric constant, so the two concepts are the same in this study).

The velocity of the TDR waveform in each phase can also be described by

$$v_i = \frac{2l_i}{t_i}, \quad (4)$$

where i represents s, w, or a.

Combining Eqs. (2)–(4) leads to the equation:

$$l\sqrt{K_a} = l_s\sqrt{K_s} + l_w\sqrt{K_w} + l_a\sqrt{K}. \quad (5)$$

Define α as the value of the volume of the air phase compared to the volume of the solid phase, i.e.,

$$\alpha = \frac{l_a}{l_s}, \quad (6)$$

the gravimetric water content of soil can be expressed as

$$w = \frac{l_w}{G_s l_s}, \quad (7)$$

where G_s is the specific gravity of the solids.

Combining Eq. (1) and Eqs. (5)–(7) leads to Eq. (8):

$$w = \frac{-\sqrt{K} - \frac{\sqrt{K_s} - \sqrt{K}}{1+\alpha} + \sqrt{K_a}}{G_s \frac{\sqrt{K_w}}{1+\alpha} - \frac{G_s}{1+\alpha} \sqrt{K_a}}. \quad (8)$$

As shown in Eq. (8), the relationship between gravimetric water content and apparent dielectric constant is influenced by the dielectric constant of each component, the value of α , etc. For most natural soils, the typical values of the dielectric constants of the solid phase K_s , air phase K , and liquid phase K_w are 3–5, 1, and 81, respectively. The value of the specific gravity of solids G_s is usually around 2.6–2.8. The parameters above can be regarded as constants. According to the definition of α , α is less than the void ratio, which varies between 0.4–1.5 (Chen Y., 2011). Then, the range of $1/(1+\alpha)$ is limited between 0.4–0.71, which represents the degree of compaction of the soil. Therefore, it assumes that treating $1/(1+\alpha)$ as constant will introduce negligible errors for soils of different types and compaction. From Eq. (8), the calibration equation between gravimetric water content and apparent dielectric constant has the form of

$$w = \frac{A + \sqrt{K_a}}{B + C\sqrt{K_a}}, \quad (9)$$

where A , B , and C are empirical constants. Values of them are obtained through regression analysis of real tests.

3 Data collection

To obtain the parameters in Eq. (9) through regression analysis, a total of 418 data points from TDR tests of 45 soil samples from our experiments and from the literature were collected.

3.1 Data from laboratory test

Three types of soils commonly used in engineering were used to conduct the laboratory tests: Fujian sand, Qiantang silt, and one clay soil. The

properties of these soils, including the specific gravity of solids (G_s), liquid limit (L_L), plasticity index (P_I), grain-size distributions, and the classified types by the Unified Soil Classification System (USCS) are shown in Table 1.

Table 1 Physical properties of soils in the laboratory tests

Soil	USCS type	G_s	L_L	P_I	GSD (%)		
					Sand	Silt	Clay
Fujian sand	SP	2.64	–	–	100.0	0	0
Qiantang silt	ML	2.69	31.7	9.1	11.2	83.0	5.8
Clay sand	CL	2.66	48	15.4	11.4	51.9	36.7

SP: poorly graded sand soils; ML: silt soils; CL: low plasticity clay soils; GSD: grain-size distribution

The TDR tests were conducted with the Campbell Scientific TDR 100 apparatus and its PCTDR software. The TDR waveforms are analyzed to obtain the dielectric constant (Baker and Allmaras, 1990). The probe used here is the same as the one recommended by ASTM (2012a) with a height of 116 mm for the compact mold.

Before the test, the soil samples were oven dried, pulverized, sieved first, and mixed with tap water to approach the targeted water contents. After being placed in a room with a constant temperature around 20 °C in a sealed plastic bag for 24 h, the soil samples were then compacted in the mold according to ASTM (2012b) and a TDR test was performed. Finally the soil samples were oven dried to obtain the real gravimetric water content.

Through this method, the dielectric constant and the dry density of the sand, silt, and clay soil samples with seven, seven, and six different water contents, respectively, were obtained.

3.2 Data from the field test

Based on the pavement maintenance project of an airport in the west of China, TDR tests for soils of different depths at the pavement area and soil surface area were performed. Soil samples at targeted depths were taken by the dry drilling method. Part of the soil was sealed in plastic bags and tested by oven dry method while the rest was compacted in the mold for a TDR test. The properties of field soils are shown in Table 2.

As the test condition in the field was complex and time was limited, twelve data points were obtained.

Table 2 Physical properties of the field soils

Soil	USCS type	G_s	L_L	P_I	GSD (%)		
					Sand	Silt	Clay
Airport-1	CL	2.61	47.3	33.4	7.1	74.0	18.9
Airport-2	CL	2.64	35.9	11.8	15.6	65.2	19.2
Airport-3	CL	2.75	35.1	14.9	23.8	60.7	15.5

3.3 Data from the literature

Information from the literature on 39 soil samples and 386 TDR data points, including the values of dielectric constant, dry density, and gravimetric water content by the oven dry method, was used for re-analysis in this study. In this database, the number of samples for sandy soils, silt soils, and clay soils are 9, 8, and 22, respectively. The range of gravimetric water content is 0–0.55 g/g and dry density is 1.07–2.3 g/cm³. Temperature varies from 4 °C to 40 °C. Properties of soils from the literature are shown in Table 3.

According to their properties, the soils collected are classified into four types: sandy soils (S), silt soils (ML), low plasticity clay soils (CL) and high plasticity clay soils (CH). According to USCS, S soils include SW, SP, SM, SM-SC, and SM-SW soil types. CL soils include CL and CL-ML soil types. CH soils include CH and CH-CL soil types. The compaction energy levels used in Jung (2011) on ASTM Reference Soils (ASTM, 2010) were 360, 600, and 2700 kJ/m³ for compaction methods of reduced compaction (RC), standard compaction (SC), and modified compaction (MC), respectively (ASTM, 2007; 2009). Lin (1999) used the same compaction methods to conduct a compaction test for M1–M5 soils. The values of electrical conductivity used by Jung (2011) to perform a TDR test on ASTM Reference Soils (ASTM, 2010) are about 62 mS/m for tap water and 130 mS/m for saline water.

4 Results and discussion

4.1 Regression analysis

The data base collected above was used to obtain the parameters in Eq. (9) through regression analysis. As shown in Fig. 2 (p.245), ten points labeled with squares show great disparity, and these

Table 3 Properties of the soils from literatures

Soil	USCS type	G_s	L_L	P_1	GSD (%)			Compaction method	Temperature (°C)	Pore fluid	NDS	Reference
					Sand	Silt	Clay					
A1	SW	–	–	–	–	–	–	SC	Room	CaCl ₂	6	Chen <i>et al.</i> (2014)
A2	ML	–	–	–	–	–	–	SC	Room	CaCl ₂	7	Chen <i>et al.</i> (2014)
A3	CL	–	–	–	–	–	–	SC	Room	CaCl ₂	7	Chen <i>et al.</i> (2014)
Silt	ML	2.67	–	–	5.7	93.7	0.6	SC	Room	TAP	6	Xu (2008)
Xiaoshan clay	CL	2.69	34	23	0	69.6	30.4	SC	Room	TAP	5	Xu (2008)
Bentonite	CH	2.82	60	45	29.5	39.8	30.7	SC	Room	TAP	5	Xu (2008)
Crosby till	CL-ML	–	41	23	16	50	34	SC	4, 10, 20, 30, 40	TAP	31	Drnevich <i>et al.</i> (2001)
Kaolinite	CL	–	30	6	0	0	100	SC	4, 20, 30	TAP	8	Drnevich <i>et al.</i> (2001)
Illite	CH-CL	–	50	28	0	0	100	SC	4, 20, 30	TAP	7	Drnevich <i>et al.</i> (2001)
Concrete sand	SW	–	–	–	100	0	0	SC	4, 20, 40	TAP	6	Drnevich <i>et al.</i> (2001)
Fine sand	SP	2.65	–	–	100	0	0	SC	4, 20, 40	TAP	6	Drnevich <i>et al.</i> (2001)
Houston clay	CH	–	54	31	0	5*	95*	SC	4, 10, 20, 30, 40	TAP	30	Drnevich <i>et al.</i> (2001)
Grade sand	SW	–	–	–	–	–	–	SC	Room	TAP	4	Yu and Drnevich (2004)
Silt	ML	2.69	32	9	11.2	83.0	5.8	SC	Room	–	5	Chen W. (2011)
Silt sand	SM	2.66	28.1	5	–	–	–	SC	Room	–	6	Chen W. (2011)
Silt	ML	2.68	28.9	6	–	–	–	SC	Room	–	5	Chen W. (2011)
Mucky soil	CL	2.73	33.4	14.9	–	–	–	SC	Room	–	7	Chen W. (2011)
Silt clay	CL-ML	2.72	30.3	13.4	–	–	–	SC	Room	–	7	Chen W. (2011)
Vigo	CL	–	36	12	–	–	–	SC, MC	Room	–	9	Feng <i>et al.</i> (1998)
Hendricks I	CL	–	37	13	–	–	–	SC, MC	Room	–	5	Feng <i>et al.</i> (1998)
Indianapolis	ML	–	15	–	–	–	–	SC, MC	Room	–	9	Feng <i>et al.</i> (1998)
Hendricks II	CL	–	32	11	–	–	–	SC, MC	Room	–	10	Feng <i>et al.</i> (1998)
Bloomington	CL-CH	–	50	24	–	–	–	SC, MC	Room	–	9	Feng <i>et al.</i> (1998)
ASTM-CH	CH	2.72	59.8	39.2	1.2	42.5	56.3	SC, MC, RC	20	TAP, DI, SAL	29	Jung (2011)
ASTM-CL	CL	2.67	33.4	13.6	11.5	42.5	46	SC, MC, RC	20	TAP	15	Jung (2011)
ASTM-ML	ML	2.73	27.4	4.1	1	94	5	SC, MC, RC	20	TAP	15	Jung (2011)
ASTM-SP	SP	2.66	–	–	99	–	–	SC, MC, RC	20	TAP	19	Jung (2011)
M1	SM-SC	2.76	–	–	55	35	10	SC, MC, RC	20	TAP	12	Jung (2011)
M2	ML	2.77	16.2	5.7	37.5	45.0	17.5	SC, MC, RC	20	TAP	14	Jung (2011)
M3	CL	2.83	28.5	16.2	20	55	25	SC, MC, RC	20	TAP	12	Jung (2011)
M4	CL	2.83	33.7	14.8	12.5	47.5	40	SC, MC, RC	20	TAP	13	Jung (2011)
M5	CL	2.82	41	21.1	5	40	55	SC, MC, RC	20	TAP	13	Jung (2011)
GRP	CL	2.68	31.1	15.8	12	60.5	27.5	–	21–23	–	6	Jung (2011)
SAG	SM-SW	2.72	–	–	65.3	16.4	9	–	21–23	–	5	Jung (2011)
DFA	CL-ML	2.62	17.6	5.3	35	50.4	14.6	–	21–23	–	5	Jung (2011)
Poor sand	SP	–	–	–	98.7	1.34	–	SC, MC	–	–	11	Rathje <i>et al.</i> (2006)
Taylor clay	CH	–	–	–	4.17	95.8	–	SC	–	–	5	Rathje <i>et al.</i> (2006)
Clay	CL	–	–	–	12.2	77.9	–	SC	–	–	6	Rathje <i>et al.</i> (2006)
Sandy clay	CH	–	–	–	37.2	62.9	–	SC	–	–	6	Rathje <i>et al.</i> (2006)

SW: well-graded sand soils; SM: silt sand soils; NDS: number of data samples; TAP: tap water; DI: deionized water; SAL: saline water (about twice pore fluid conductivity of TAP); *: the data is estimated; Room: room temperature, usually around 20 °C

points were omitted during data fitting. At the same time, only the data points at 20 °C given by Drnevich *et al.* (2001) were adopted for the regression analysis. It should be pointed out that the subsequent statistical analysis covered the ten omitted scattered points. The results of regression analysis are shown in Fig. 2 and the empirical calibration equation is given in Eq. (10). The correlation coefficient R^2 is 0.9019, showing a good correlation between the regression results and the data points.

$$w = \frac{-1.4637 + \sqrt{K_a}}{22.1373 - 1.4606\sqrt{K_a}} \quad (10)$$

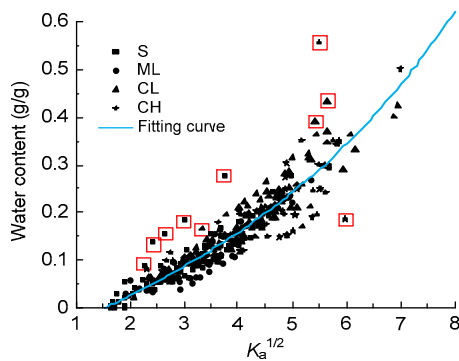


Fig. 2 Result of regression analysis by Eq. (10)

Eq. (10) ignores the influence of soil type, dry density, compaction energy, pore fluid conductivity, temperature, etc. To quantify and evaluate the effect of these elements on the accuracy of Eq. (10), statistical quantitative evaluation indexes are adopted as follows: (1) errors (Δw) reflecting the difference between water contents calculated by Eq. (10) and by the oven dry method; (2) average errors (E); (3) standard errors (SE); (4) standard errors of estimate (SEE). E is used to evaluate the degree of deviation of the calculated result from the real value and SE indicates the discrete extent of E . SEE estimates the dispersion of the overall errors (Jung, 2011). The definitions of each parameter are shown as follows:

$$\Delta w = w - w_o, \quad (11)$$

$$E = \frac{\sum_1^N |\Delta w|}{N}, \quad (12)$$

$$SE = \sqrt{\frac{\sum_1^N (\Delta w - E)^2}{N}}, \quad (13)$$

$$SEE = \sqrt{\frac{\sum_1^N (\Delta w)^2}{N - 2}}, \quad (14)$$

where w_o is gravimetric water content obtained by the oven dry method and N is the number of data points.

4.2 Soil type effects

Error variations along with water content for different soil types are shown in Fig. 3. Values of E , SE, SEE, and the distribution of errors are shown in Table 4.

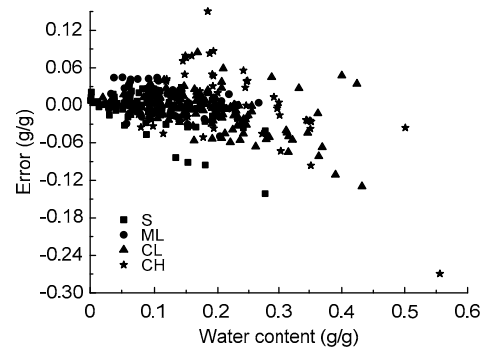


Fig. 3 Errors of water content by Eq. (10) vs. gravimetric water content for different soil types

As shown in Fig. 3, errors tend to increase with the increasing of water content. The reason is that the values of A , B , and C are assumed constant in the proposed Eq. (9) and obtained through regression analysis, which means the variation of water content with dry density is ignored.

As shown in Table 4, the values of statistic parameters for S, ML, and CL soils are small and the ratio of errors within ± 0.03 g/g are large, which indicates that Eq. (10) has a good accuracy for these soils. But for CH soils, it shows a relatively poor result, which can be attributed to ignorance of the effect of bound water for soils with high clay contents. In general, errors of most data points of all soil types are within ± 0.05 g/g.

4.3 Dry density effects

Errors varying with dry density are shown in Fig. 4. Values of E , SE, SEE, and the distribution of errors are shown in Table 5.

As shown in Fig. 4, errors show a relatively obvious dependency on the change of dry density. With

the increasing of dry density, errors gradually vary from negative to positive, meaning that Eq. (10) underestimates the result in cases of low dry density and overestimates the result in cases of high dry density. Note that errors are large for low dry density soils. The reason can also be attributed to the neglect of variation of A , B , and C .

As shown in Table 5, when dry density ranges around 1.3–2.3 g/cm³, the statistic parameters E and SEE are around 0.01–0.03 g/g and errors within ±0.05 g/g are at a high level, indicating that Eq. (10) has a good accuracy for a wide scope of dry density

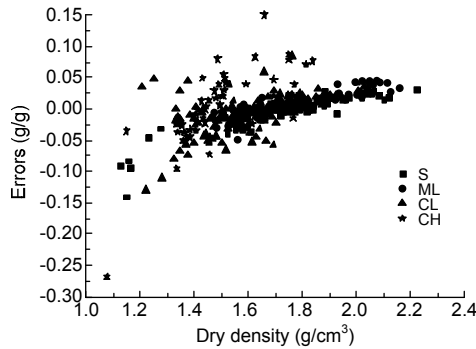


Fig. 4 Errors of water content by Eq. (10) vs. dry density

cases. However, for dry density of 1.0–1.3 g/cm³, it shows a poor result, which indicates that at this condition, the effect of dry density on the result of Eq. (10) cannot be ignored. Therefore, it is necessary to correct Eq. (10) to consider the effect of dry density.

In Eq. (9), parameters A , B , and C are assumed to be constants. Actually, they are variables and have a relationship with dry density, etc. Herein, values of A , B , and C are assumed to have a simple linear relation with dry density and then Eq. (9) becomes

$$w = \frac{\left(a \frac{\rho_d}{\rho_w} + b \right) + \sqrt{K_a}}{\left(c \frac{\rho_d}{\rho_w} + d \right) + \left(f \frac{\rho_d}{\rho_w} + g \right) \sqrt{K_a}}, \quad (15)$$

where a , b , c , d , f , and g are modified constants, ρ_d is the dry density, and ρ_w is the density of water.

The data points collected in this research are used to make regression analysis of the parameters in Eq. (15). The result empirical calibration equation is given in Eq. (16). The value of R^2 is 0.9413.

Table 4 Errors for water content by Eq. (10) vs. different soil types

Soil type	Number of data samples	E	SE	SEE	Distribution of error (%)				
					≤±0.01 g/g	≤±0.02 g/g	≤±0.03 g/g	≤±0.04 g/g	≤±0.05 g/g
S	74	0.019	0.023	0.031	40.5	66.2	83.8	91.9	94.6
ML	68	0.017	0.013	0.022	39.7	67.7	83.8	89.7	100.0
CL	148	0.022	0.022	0.031	34.5	62.2	75.7	81.8	89.2
CH	62	0.036	0.041	0.056	22.6	37.1	56.5	71.0	80.7

For distribution of error, the ‘≤±0.01’ column represents the percentage of errors within ±0.01 g/g, the meanings of the rest columns are analogous, and the same below

Table 5 Errors for water content by Eq. (10) vs. different dry densities

Density (g/cm ³)	Number of data samples	E	SE	SEE	Distribution of error (%)				
					≤±0.01 g/g	≤±0.02 g/g	≤±0.03 g/g	≤±0.04 g/g	≤±0.05 g/g
1.0–1.3	12	0.093	0.065	0.124	0.00	0.00	0.00	25.00	41.70
1.3–1.4	25	0.036	0.024	0.045	4.00	28.00	52.00	64.00	76.00
1.4–1.5	40	0.028	0.02	0.035	20.00	45.00	62.50	70.00	87.50
1.5–1.6	72	0.018	0.015	0.024	37.50	63.90	79.20	90.30	95.80
1.6–1.7	62	0.018	0.025	0.031	56.50	75.80	82.30	87.10	90.30
1.7–1.8	63	0.013	0.018	0.022	57.10	81.00	93.70	95.20	95.20
1.8–1.9	35	0.015	0.016	0.023	4.00	82.90	91.40	94.30	94.30
1.9–2.0	14	0.021	0.009	0.025	7.14	57.10	85.70	92.90	100.00
2.1–2.3	29	0.029	0.009	0.032	0.00	13.80	58.60	75.90	100.00

w=

$$\frac{0.3039 \frac{\rho_d}{\rho_w} - 2.1851 + \sqrt{K_a}}{\left(18.0283 \frac{\rho_d}{\rho_w} - 17.9531\right) + \left(-0.6806 \frac{\rho_d}{\rho_w} + 1.8351\right) \sqrt{K_a}} \quad (16)$$

Values of *E*, *SE*, *SEE*, and the distribution of errors after considering the effect of dry density by Eq. (16) are shown in Table 6.

Through Table 6, statistic parameters of soil samples with 1.0–1.3 g/cm³ dry densities show obvious improvement. In reality, if information on dry density is available, Eq. (16) can offer a better accuracy.

Considering total density ρ_t , which is more available in geotechnical engineering practice, Eq. (16) can be combined with the following equation:

$$\rho_d = \frac{\rho_t}{1 + w} \quad (17)$$

Combining Eq. (16) and Eq. (17), we can obtain the following equation:

$$w = \frac{-f_2 + \sqrt{f_2^2 - 4f_1f_3}}{2f_1} \quad (18)$$

where

$$f_1 = -17.9531 + 1.8351\sqrt{K_a},$$

$$f_2 = 18.0283\rho_t - 0.6806\rho_t\sqrt{K_a} + 0.8351\sqrt{K_a} - 15.768,$$

$$f_3 = -0.3039\rho_t - \sqrt{K_a} + 2.1851.$$

Eq. (18) can offer the same accuracy as Eq. (16). As it is expressed in terms of total density, it can be more practical.

4.4 Compaction energy effects

For a unit soil with certain water content, when compacted with different compaction energy levels, the sample obtained will have different structures and compactness, which will influence the dielectric constant by the TDR test. For brevity, only comparisons of water content by Eq. (10) and the oven dry method for ASTM-CH soil are plotted in Fig. 5. Values of *E*, *SE*, *SEE*, and the distribution of errors for soil samples of ASTM Reference Soils and M1–M5 soils with three compaction energy levels are shown in Table 7.

Fig. 5 and Table 7 show that, with the compaction energy increasing, *E* and *SEE* have an increasing trend and the ratios of errors within ± 0.03 g/g and ± 0.05 g/g decrease correspondingly. This means

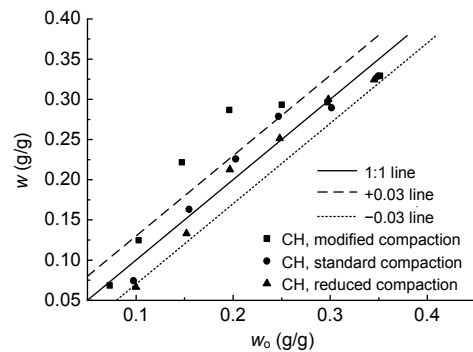


Fig. 5 Comparisons of water content by Eq. (10) and the oven dry method for ASTM-CH soil at different compaction energy levels

Table 6 Errors for water content by Eq. (16) vs. different dry densities

Density (g/cm ³)	Number of data samples	<i>E</i>	<i>SE</i>	<i>SEE</i>	Distribution of error (%)				
					$\leq \pm 0.01$ g/g	$\leq \pm 0.02$ g/g	$\leq \pm 0.03$ g/g	$\leq \pm 0.04$ g/g	$\leq \pm 0.05$ g/g
1.0–1.3	12	0.029	0.021	0.039	33.30	41.70	50.00	75.00	91.70
1.3–1.4	25	0.020	0.018	0.028	48.00	52.00	68.00	88.00	92.00
1.4–1.5	40	0.026	0.021	0.035	25.00	40.00	67.50	80.00	90.00
1.5–1.6	72	0.016	0.014	0.021	48.60	75.00	83.30	93.10	97.20
1.6–1.7	62	0.017	0.017	0.025	45.20	64.50	82.30	88.70	93.60
1.7–1.8	63	0.009	0.008	0.012	66.70	90.50	96.80	100.00	100.00
1.8–1.9	35	0.007	0.007	0.010	77.10	94.30	97.10	100.00	100.00
1.9–2.0	14	0.008	0.004	0.010	71.40	100.00	100.00	100.00	100.00
2.1–2.3	29	0.009	0.007	0.011	69.00	93.10	100.00	100.00	100.00

that the errors of Eq. (10) increase with the increasing of compaction energy.

4.5 Pore fluid conductivity effects

Soils with different pore fluid conductivities will cause different energy losses under the TDR waveform, which will influence the value of their dielectric constants. Comparisons of water content by Eq. (10) and the oven dry method for ASTM-CH soil are shown in Fig. 6. Values of E , SE, SEE, and the distribution of errors for ASTM-CH soil with different pore fluid conductivity are shown in Table 8.

As shown in Fig. 6 and Table 8, with the pore fluid conductivity increasing, the statistical parameters E and SEE have an increasing trend and the ratio of errors within ± 0.03 g/g and ± 0.05 g/g decrease

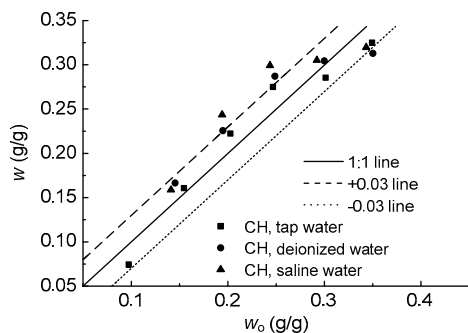


Fig. 6 Comparisons of water content by Eq. (10) and the oven dry method for ASTM-CH soil at different pore fluid conductivities

correspondingly, which indicates that errors of Eq. (10) increase with the increasing of pore fluid conductivity. Note that the parameters for soil with deionized water as pore fluid seem not to be in accordance with the conclusion above, which may be because the constants in Eq. (10) are obtained mainly from soil samples with tap water as the pore fluid. In addition, as the samples are small and the salinity is at a low level, the conclusions above need to be further discussed.

4.6 Temperature effects

Temperature has a different effect on the value of dielectric constant for different soil types (Wraith and Or, 1999; Robinson *et al.*, 2003; Schanz *et al.*, 2011). Pepin *et al.* (1995) and Persson and Berndtsson (1998) found that for sandy soils, dielectric constant decreases with the increasing of temperature. Drnevich *et al.* (2001) pointed out that due to a large content of clay particles, the dielectric constant of cohesive soils increases with the increasing of temperature.

Drnevich *et al.* (2001) performed TDR tests on cohesive and cohesionless soils with temperatures ranging from 4 °C to 40 °C. For brevity, only comparisons of water content by Eq. (10) and the oven dry method for Crosby Till soil are plotted in Fig. 7. Values of E , SE, SEE, and the distribution of errors for soil samples with different temperatures are shown in Table 9.

Table 7 Errors for water content by Eq. (10) at different compaction energy levels

Compaction method	Number of data samples	E	SE	SEE	Distribution of error (%)				
					$\leq \pm 0.01$ g/g	$\leq \pm 0.02$ g/g	$\leq \pm 0.03$ g/g	$\leq \pm 0.04$ g/g	$\leq \pm 0.05$ g/g
MC	45	0.020	0.017	0.027	28.90	57.80	77.80	91.10	95.60
SC	43	0.013	0.008	0.016	34.90	74.40	100.00	100.00	100.00
RC	44	0.011	0.009	0.014	56.80	77.30	97.70	100.00	100.00

Table 8 Errors for water content by Eq. (10) at different pore fluid conductivities

Pore fluid	Number of data samples	E	SE	SEE	Distribution of error (%)				
					$\leq \pm 0.01$ g/g	$\leq \pm 0.02$ g/g	$\leq \pm 0.03$ g/g	$\leq \pm 0.04$ g/g	$\leq \pm 0.05$ g/g
DI	5	0.026	0.013	0.038	20.00	20.00	40.00	100.00	100.00
TAP	6	0.020	0.007	0.026	16.70	50.00	100.00	100.00	100.00
SAL	5	0.032	0.017	0.047	0.00	40.00	60.00	60.00	80.00

DI: deionized water; TAP: tap water; SAL: saline water (about twice pore fluid conductivity of TAP). For distribution of error, the ' $\leq \pm 0.01$ ' column represents the percentage of errors within ± 0.01 g/g, and the meanings of the rest columns are analogous

As shown in Fig. 7 and Table 9, the statistical parameters E and SEE are relatively large, which is mainly due to the presence of some scattered data, specifically, the data points above 0.4 g/g with a low dry density. Errors trend to decrease with the increasing in temperature. When the temperature is within 4–30 °C, it has no significant influence on the result of Eq. (10). Drnevich *et al.* (2001) pointed out that the effect of temperatures from 5 °C to 20 °C can be ignored since their influence is small. ASTM (2012a) presents the equations used to modify the effect of temperature.

4.7 Comparison of Topp’s equation and ‘two-step method’ equation

Topp *et al.* (1980) proposed a widely used empirical calibration equation:

$$\theta = 4.3 \times 10^{-6} K_a^3 - 5.5 \times 10^{-4} K_a^2 + 2.92 \times 10^{-2} K_a - 5.3 \times 10^{-2}, \tag{19}$$

where θ is the volumetric water content.

Gravimetric water content (w) and volumetric water content (θ) are related as follows:

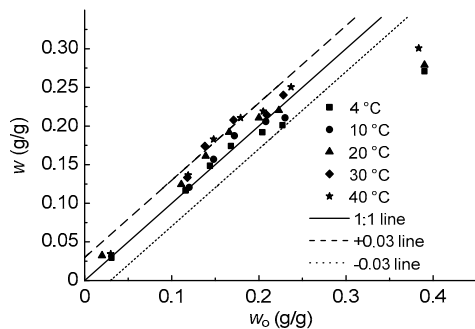


Fig. 7 Comparisons of water content by Eq. (10) and the oven dry method for Crosby Till soil at different temperatures

$$w = \frac{\rho_w}{\rho_d} \theta. \tag{20}$$

Combining Eqs. (19) and (20), the gravimetric water content calculated by Topp’s equation is obtained.

Siddiqui and Drnevich (1995) performed the ‘two-step method’ to test the water content and dry density of soil. The empirical calibration equation they developed is

$$\sqrt{K_a} \frac{\rho_w}{\rho_d} = a_1 + b_1 w, \tag{21}$$

where a_1 and b_1 are calibration constants.

Drnevich *et al.* (2005) pointed out that although calibration constants in Eq. (21) are soil-dependent, uniform calibration constants can be used within acceptable limits for engineering practice. Herein, the values of a_1 and b_1 are 1 and 8.5 for cohesionless soils and 0.95 and 8.8 for cohesive soils, respectively, as recommended by Drnevich *et al.* (2005).

Comparisons of water content calculated by Eq. (10), Topp’s equation, Eq. (21), and Eq. (16) are shown in Table 10.

As shown in Table 10, results calculated by Topp’s equation, Eq. (21), and Eq. (10) are very close. Compared with Topp’s equation which expresses water content in volumetric form and the ‘two-step method’ equation which considers the influence of dry density, the statistical parameters E and SEE of Eq. (10) are slightly larger than those in the two other methods. After being modified by considering the dry density effects, Eq. (16) has a better accuracy.

Through the analysis above, Eq. (10) shows a good accuracy for most of soil types and a slight influence by a wide range of dry densities, compaction energy levels, and pore fluid conductivity, at

Table 9 Errors for water content by Eq. (10) at different temperatures

Temperature (°C)	Number of data samples	E	SE	SEE	Distribution of error (%)				
					$\leq \pm 0.01$ g/g	$\leq \pm 0.02$ g/g	$\leq \pm 0.03$ g/g	$\leq \pm 0.04$ g/g	$\leq \pm 0.05$ g/g
4	23	0.056	0.066	0.090	21.70	34.80	47.80	56.50	65.20
10	11	0.053	0.080	0.106	27.30	54.60	72.70	72.70	72.70
20	22	0.048	0.061	0.081	22.70	40.90	63.60	68.20	68.20
30	15	0.049	0.065	0.087	26.70	53.30	53.30	66.70	66.70
40	17	0.034	0.047	0.062	23.50	58.80	70.60	82.40	82.40

Table 10 Comparison of water content obtained by Eq. (10), Topp's equation, Eq. (21), and Eq. (16)

Equation	Number of data samples	E	SE	SEE	Distribution of error (%)				
					$\leq \pm 0.01$ g/g	$\leq \pm 0.02$ g/g	$\leq \pm 0.03$ g/g	$\leq \pm 0.04$ g/g	$\leq \pm 0.05$ g/g
Eq. (10)	352	0.023	0.026	0.035	34.70	59.70	75.60	83.50	90.90
Topp's equation (Topp et al., 1980)	352	0.020	0.018	0.027	29.60	65.30	81.50	89.20	93.20
Eq. (21)	352	0.018	0.021	0.028	45.50	75.60	82.70	88.10	92.30
Eq. (16)	352	0.015	0.016	0.022	53.40	73.60	84.90	92.60	96.30

temperatures commonly encountered in engineering practice.

5 Conclusions

From the database including 45 kinds of soil samples and 418 data points, an empirical calibration equation has been developed to relate gravimetric water content directly with apparent dielectric constant. The main conclusions are as follows:

1. The accuracy of the new empirical calibration equation is within ± 0.05 g/g for commonly encountered soils.

2. The new empirical calibration equation underestimates the result in low dry density and overestimates the result in high dry density. For dry density ranging between 1.3 and 2.3 g/cm³, the new empirical calibration equation shows a good accuracy.

3. Errors of the new empirical calibration equation tend to increase with the increasing of compaction energy and pore fluid conductivity. However, for the commonly encountered ranges of compaction energy and pore fluid conductivity, the new empirical calibration has a good accuracy.

4. Temperature has no sensible influence on the results from the new empirical calibration equation when it is used within 4–30 °C.

5. This empirical calibration equation can be used to measure water content by the TDR method conveniently and efficiently in engineering practice without calibration.

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中文概要

题目: 土体含水率时域反射法现场测试经验模型研究

目的: 建立含水率与介电常数间的经验关系模型是利用时域反射 (TDR) 技术测试土体含水率的关键。通过收集并建立包含 45 种土样 418 个试验数据点的数据库, 提出一个土体质量含水率与表观介电常数间的经验公式, 分析经验公式误差随土体类型、干密度、击实功、孔隙水电导率和温度等因素的变化规律, 并提出考虑干密度影响的修正方法。

创新点: 1. 基于电磁波相互作用理论, 直接建立土体质量含水率和介电常数间的关系模型; 2. 通过数据拟合得到通用型经验公式, 可在现场无标定快速高效地实现含水率测试。

方法: 1. 通过理论分析, 直接建立土体质量含水率和介电常数间的关系模型 (公式 (9)); 2. 通过试验数据收集和回归分析得到通用型经验公式 (图 2 和公式 (10)); 3. 通过影响因素分析, 对公式的适用性和有效性进行分析 (图 3~7)。

结论: 1. 该经验公式对常见的土体类型均能给出误差在 ± 0.05 g/g 以内的结果; 2. 在 $1.3\sim 2.3$ g/cm³ 的干密度范围内, 该经验公式具有较好的适用性; 在工程中常见的击实功和孔隙水电导率变化范围内, 含水率测试精度可满足工程要求; $4\sim 30$ °C 温度变化范围对本经验公式的计算结果无明显影响; 3. 利用该经验公式, 对于特殊场地, 可以不通过标定实现 TDR 现场测试, 具有较好的实用性。

关键词: 土体; 质量含水率; 时域反射法; 经验公式